


UTM UNIVERSITI TEKNOLOGI MALAYSIA **OPENCOURSEWARE**

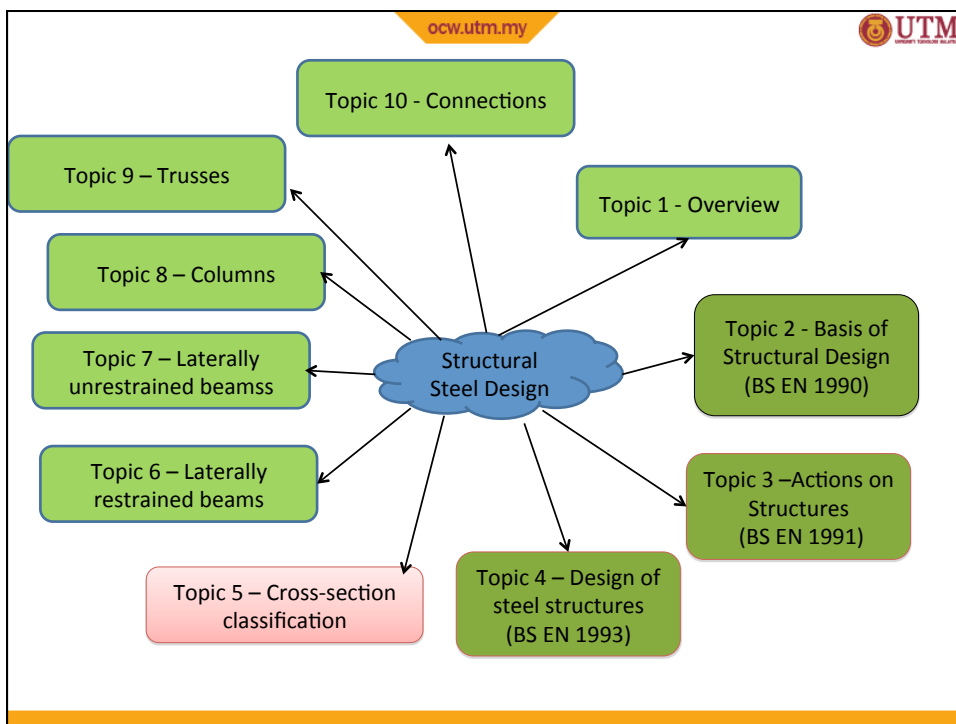
Structural Steel and Timber Design SAB3233

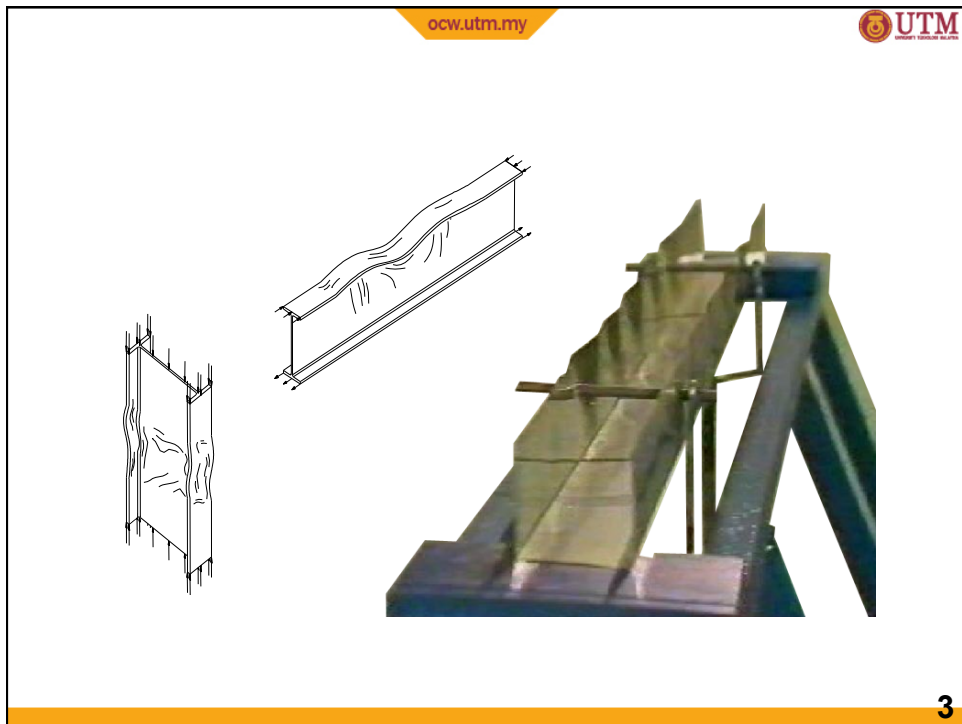
Topic 5 Cross-section classification

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Basis of section classification

Rolled or welded sections may be considered an assembly of individual plate elements

Some are outstand
- flanges of I beams
- legs of angles and Tees

Some are internal
- webs of open beams
- flanges of boxes

Rolled I-section

Hollow section

Welded box section

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Basis of section classification

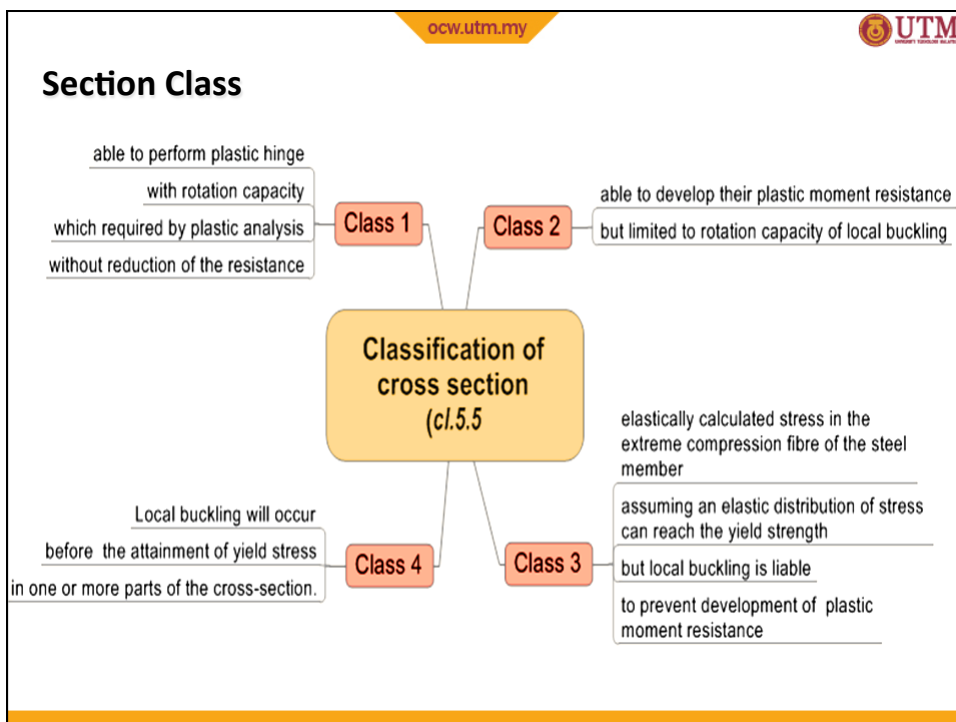
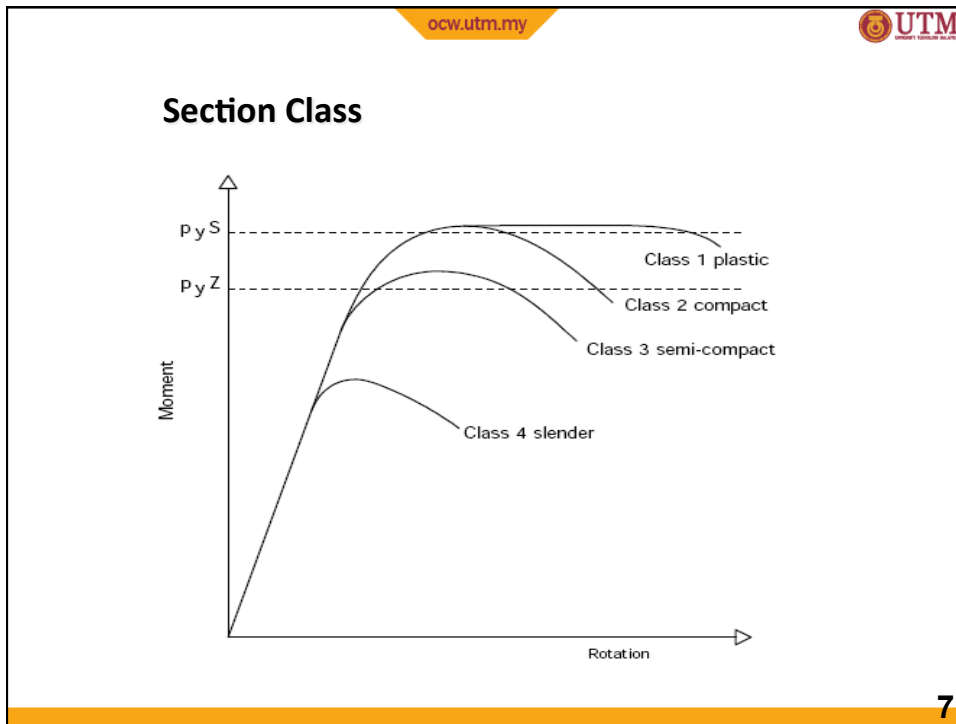
- As the plate elements are relatively thin, when loaded in compression they may buckle locally
- The tendency of any plate element within the cross section to buckle may limit the axial load carrying capacity, or the bending resistance of the section, by preventing the attainment of yield.
- Avoidance of premature failure arising from the effects of local buckling may be achieved by limiting the width-to-thickness ratio for individual elements within the cross section.

This is the basis of the section classification approach

Section Class

Classification

- ◆ EC3 defines four classes of cross section.
- ◆ The class into which a particular cross section falls depends upon
 - ◆ slenderness of each element (defined by a width-to-thickness ratio)
 - ◆ the compressive stress distribution
 - ◆ Classes are defined in terms of performance requirements for resistance of bending moments



Section Class

Classification process

Limiting width-to-thickness ratios for sections refer Table 5.2
EN 1993:11:2005

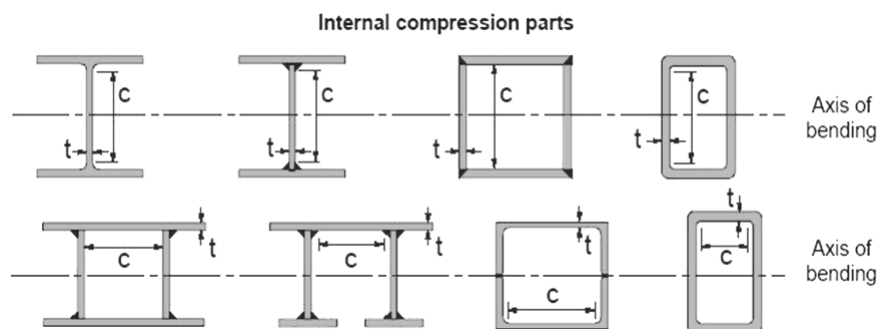
Classification process follows five basic steps

- Evaluate the slenderness ratio (c/T or d/t)
- Evaluate the parameter ϵ
- Determine the class of that element based on limiting value of thickness ratio.
- Classify the complete cross-section according to the least favorable classification

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Limiting width-to-thickness ratio

Section Class



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Limiting width-to-thickness ratio Section Class

Class	Part subject to bending	Part subject to compression	Part subject to bending and compression			
Stress distribution in parts (compression positive)						
1	$c/t \leq 72\epsilon$	$c/t \leq 33\epsilon$	when $\alpha > 0.5$: $c/t \leq \frac{396\epsilon}{13\alpha - 1}$ when $\alpha \leq 0.5$: $c/t \leq \frac{36\epsilon}{\alpha}$			
2	$c/t \leq 83\epsilon$	$c/t \leq 38\epsilon$	when $\alpha > 0.5$: $c/t \leq \frac{456\epsilon}{13\alpha - 1}$ when $\alpha \leq 0.5$: $c/t \leq \frac{41.5\epsilon}{\alpha}$			
$\epsilon = \sqrt{235/f_y}$	f_y	235	275	355	420	460
	ϵ	1.00	0.92	0.81	0.75	0.71

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Limiting width-to-thickness ratio Section Class


Outstand flanges

Rolled sections

Welded sections

Class	Part subject to compression	Part subject to bending and compression				
		Tip in compression	Tip in tension			
Stress distribution in parts (compression positive)						
1	$c/t \leq 9\epsilon$	$c/t \leq \frac{9\epsilon}{\alpha}$	$c/t \leq \frac{9\epsilon}{\alpha\sqrt{\alpha}}$			
2	$c/t \leq 10\epsilon$	$c/t \leq \frac{10\epsilon}{\alpha}$	$c/t \leq \frac{10\epsilon}{\alpha\sqrt{\alpha}}$			
$\epsilon = \sqrt{235/f_y}$	f_y	235	275	355	420	460
	ϵ	1.00	0.92	0.81	0.75	0.71

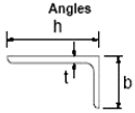
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Limiting width-to-thickness ratio


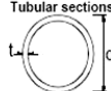
Section Class

Refer also to "Outstand flanges"
(see sheet 2 of 3)




Angles

Does not apply to angles in continuous contact with other components

Class	Section in compression																		
3	 $h/t \leq 15\epsilon$; $\frac{b+h}{2t} \leq 11.5\epsilon$																		
<p>Tubular sections</p> 																			
Class	Section in bending and/or compression																		
1	$d/t \leq 50\epsilon^2$																		
2	$d/t \leq 70\epsilon^2$																		
3	$d/t \leq 90\epsilon^2$																		
NOTE For $d/t > 90\epsilon^2$ see EN 1993-1-6.																			
$\epsilon = \sqrt{235/f_y}$	<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="width: 10%;"></th> <th style="width: 10%;">235</th> <th style="width: 10%;">275</th> <th style="width: 10%;">355</th> <th style="width: 10%;">420</th> <th style="width: 10%;">460</th> </tr> </thead> <tbody> <tr> <td style="text-align: center;">ϵ</td> <td style="text-align: center;">1.00</td> <td style="text-align: center;">0.92</td> <td style="text-align: center;">0.81</td> <td style="text-align: center;">0.75</td> <td style="text-align: center;">0.71</td> </tr> <tr> <td style="text-align: center;">ϵ^2</td> <td style="text-align: center;">1.00</td> <td style="text-align: center;">0.85</td> <td style="text-align: center;">0.66</td> <td style="text-align: center;">0.56</td> <td style="text-align: center;">0.51</td> </tr> </tbody> </table>		235	275	355	420	460	ϵ	1.00	0.92	0.81	0.75	0.71	ϵ^2	1.00	0.85	0.66	0.56	0.51
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Summary

- Structural sections may be considered as an assembly of individual plate elements.
- Plate elements may be **internal** or **outstand**
- When loaded in compression these plates may **buckle locally**
- Local buckling may **limit the load carrying capacity** of the section by **preventing the attainment of yield** strength
- Premature failure due to local buckling may be avoided by limiting the **width to thickness ratio** - or **slenderness** - of individual elements within the cross section.
- This is the basis of the **section classification** approach.
- EC3 defines four classes of cross-section.
- The class into which a particular cross-section falls depends upon the **slenderness** of each element and the compressive **stress distribution**

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